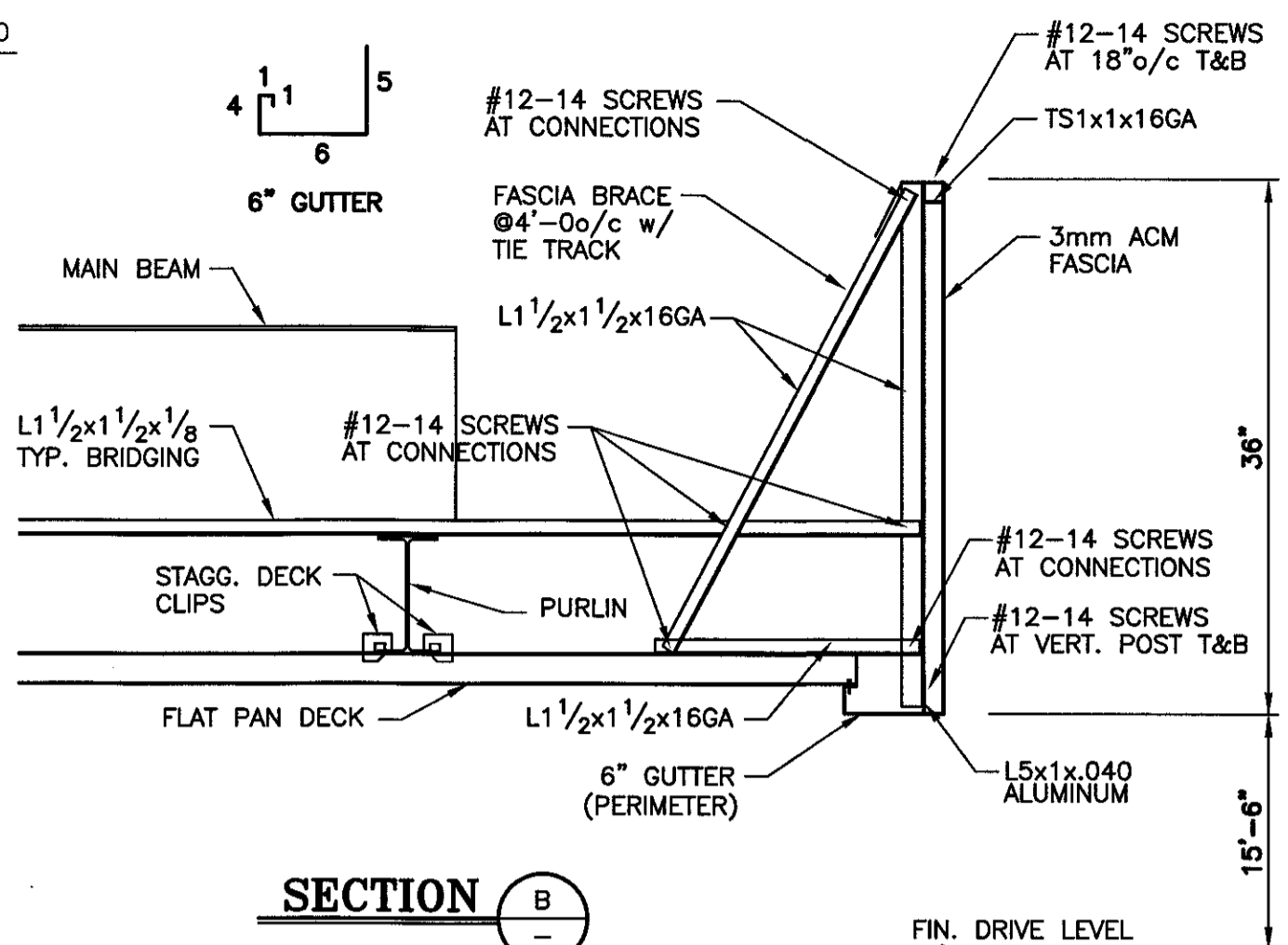
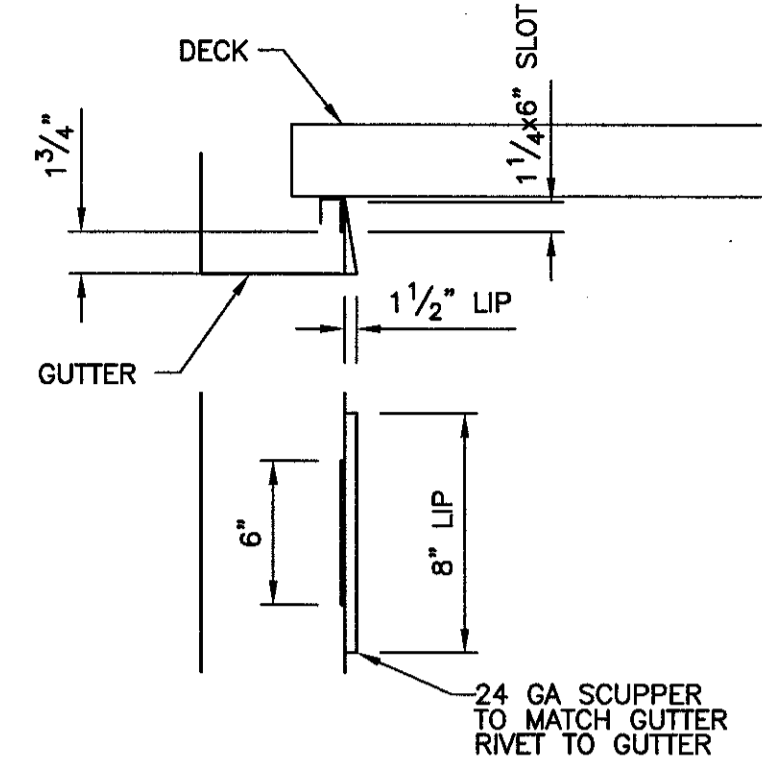


ROOF FRAMING PLAN

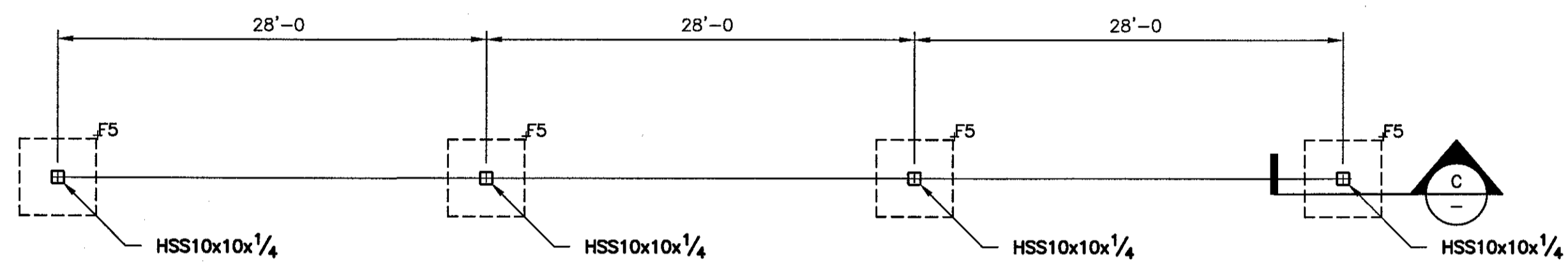
SCALE: 1/8" = 1'-0"



SECTION B



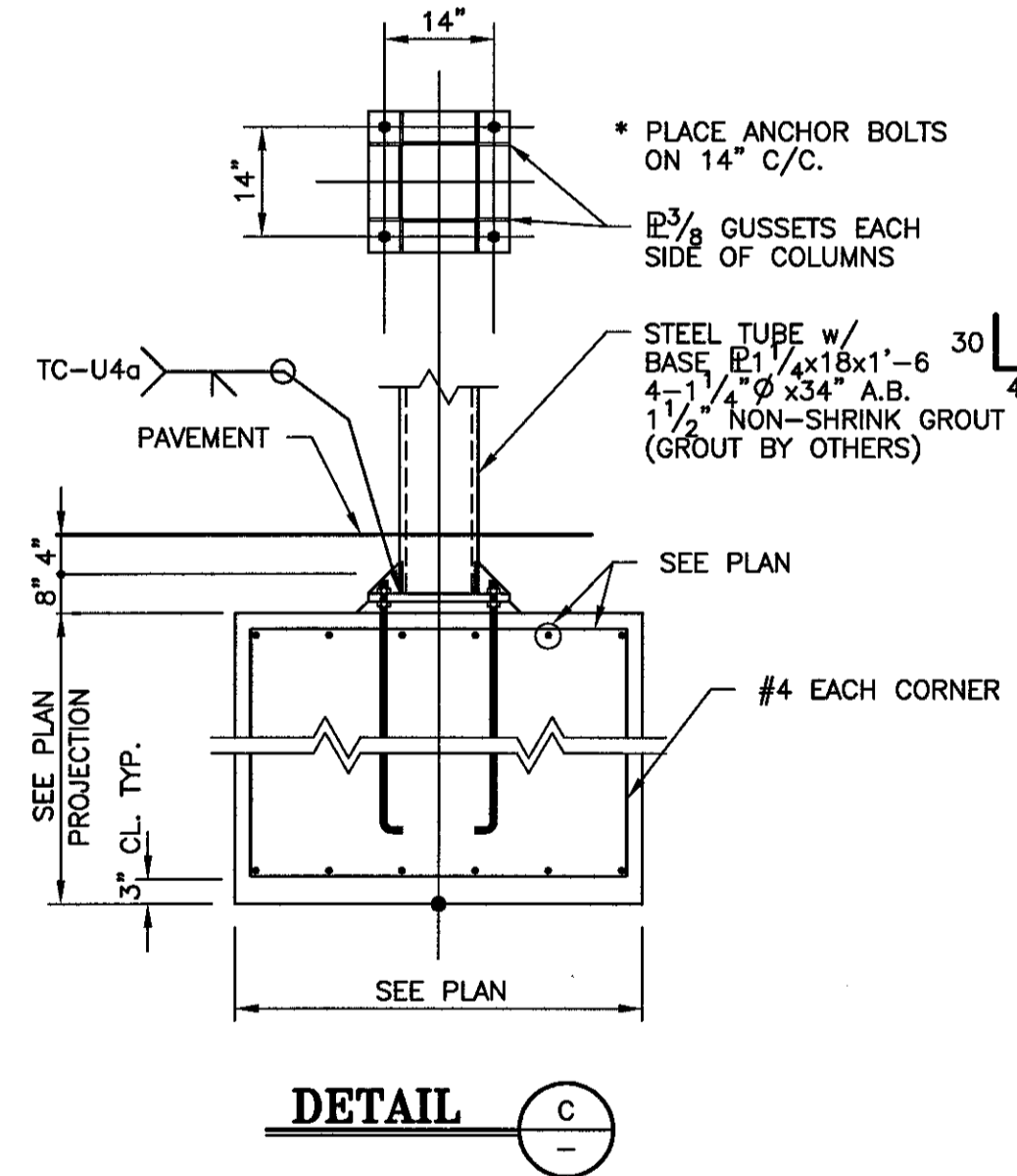
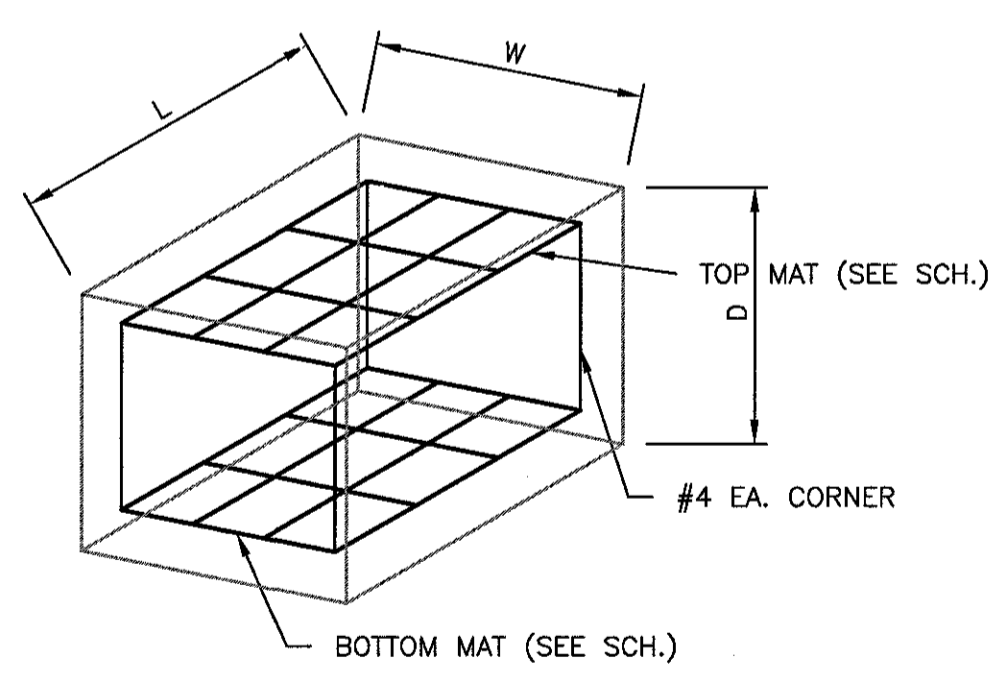
OVERFLOW



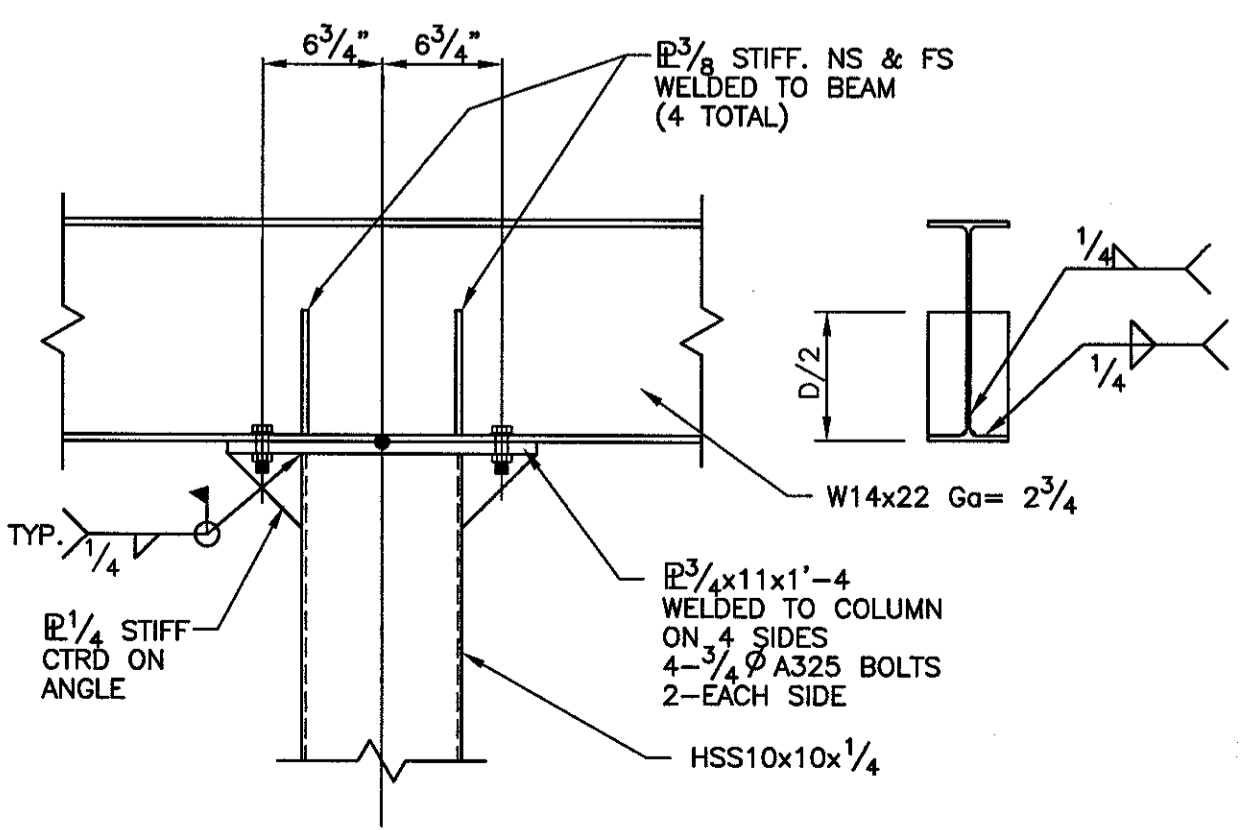
FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

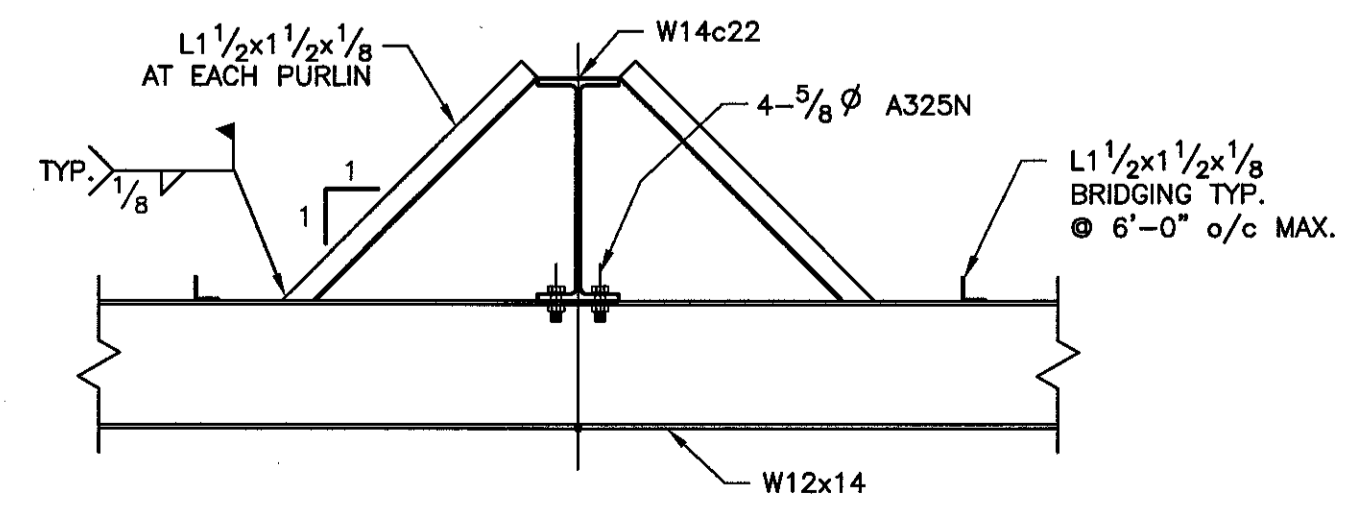
FOOTING SCHEDULE					
MARK	DIMENSIONS: LxWxD	#4 STEEL MAT TOP AND BOTTOM	#5 STEEL MAT TOP AND BOTTOM	#6 STEEL MAT TOP AND BOTTOM	REMARKS
F5	5'-0" x 5'-0" x 5'-0"	#4@8"o/c E.W.	#5@10"o/c E.W.	#6@12"o/c E.W.	#4, #5, AND #6 OPTIONS ALL STEEL SHOWN IS TO BE PLACED EACH WAY.



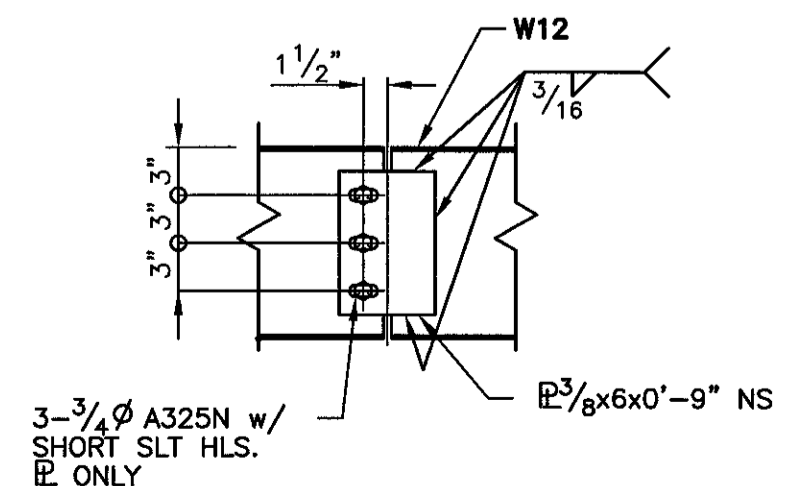
DETAIL C



SECTION D



SECTION E



SECTION F

GENERAL NOTES

FOUNDATIONS:
THE SOIL BEARING VALUE SHALL NOT BE LESS THAN 2000 PSF. THE BEARING VALUE SHALL BE VERIFIED BY THE G.C. FOOTINGS SHALL BEAR ON UNDISTURBED SOIL. ALL 17' FOOT. ELEVATIONS ARE TO BE THE SAME UNLESS SHOWN OTHERWISE. IF ELEVATIONS ARE NOT THE SAME, SHADY VENT MUST BE INSTRUCTED OF ELEVATIONS IN WRITING WHEN CANOPY ORDER IS PLACED. FOOTINGS DESIGN COMPLIES W/ACI-318.

REINFORCING STEEL:
ALL DEFORMED BARS SHALL COMPLY W/ASTM A615 FY=60 AND SHALL BE WIRE TIED AT ALL JOINTS. RUSTY, OILY, OR DIRTY STEEL SHALL NOT BE USED.

ANCHOR BOLTS:
ANCHOR BOLTS MUST BE INSTALLED WITH A TEMPLATE AND WITHIN 1/8-INCH OF MEASUREMENTS OF THE BASE PLATE OR COLUMN WILL NOT FIT. CONCRETE CONTRACTOR IS RESPONSIBLE FOR RECESSING FOOTINGS 12 - INCHES BELOW FINISH GRADE AND FOR EXTENDING ANCHOR BOLTS 8 - INCHES ABOVE FOOTINGS IN ORDER FOR CANOPY TO ERECT PROPERLY. ANCHOR BOLTS SHALL BE ASTM F1554 GRADE 36.

CONCRETE:
ALL CONCRETE SHALL BE 3000 PSI IN 28 DAYS. ALL CONC. SHALL BE PLACED IN ACCORDANCE WITH ACI-318.

STRUCTURAL STEEL:
ALL STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AISC SPECIFICATIONS. W-SHAPES SHALL BE ASTM A992. MISCELLANEOUS STEEL PLATES, ANGLES, AND CHANNELS ARE TO BE ASTM A36. HSS SHAPES SHALL BE ASTM A500 GR. C. PROOF OF WELDERS CERTIFICATION SHALL BE AVAILABLE ON REQUEST. ALL BOLTS SHALL BE IN ACCORDANCE W/ASTM A325.

OTHER CONCRETE ITEMS:
OTHER CONCRETE ITEMS SUCH AS DRIVE THRU SLAB, BUILDING SLAB, PERIMETER FOOTING, AND LOAD BEARING FOOTINGS NOT USED FOR THE CANOPY ARE TO BE AT THE SAME ELEVATION UNLESS SHOWN OTHERWISE. IF ELEVATIONS ARE TO VARY, SHADY VENT MUST BE INSTRUCTED OF ELEVATIONS IN WRITING WHEN ORDER IS PLACED.

NOTE: THIS CANOPY IS DESIGNED PER ASCE 7-16 SEE FIG. 27.3-4, UNBALANCED WIND LOAD

CASE A
1.20(20.31) .30(5.08)

CASE B
-1.10(-18.62) -1.10(-1.69)

* CLEAR WIND FLOW

NOTES:
1. THESE LOADS HAVE BEEN APPLIED TO STRUCTURE IN ACCORDANCE WITH ASCE 7-16, CHAPTER 2, 2.4.1 BASIC COMBINATIONS FOR ALLOWABLE STRESS
2. THESE STEEL MEMBERS HAVE BEEN SIZED BASED ON ASD, AISC 14th EDITION.
3. ANALYSIS OF THIS STRUCTURE HAS BEEN ACCOMPLISHED USING THE LATEST GENERATION OF MATRIX BASED SOFTWARE.
4. COLUMN SLENDERNESS FACTORS ARE BASED ON CHAPTER C, DIRECT ANALYSIS METHOD.
Kx = 1.0
Kz = 1.0
5. BASES ARE FIXED.

STRUCTURE LOADS			
PARAMETER			CODE REFERENCE IBC 2018 w/ AMENDMENTS
DEAD LOAD	4.0 PSF		1606.1
LIVE LOAD	20.0 PSF (w/ APP. RED.)		1607.12.2.1
SNOW LOAD	5.0 PSF + DRIFTS		ASCE 7-16, PART 7.0
WIND SPEED	110.0 MPH 3 SEC. GUST	18.4 PSF	ASCE 7, PART 26-29
CATEGORY II lw 1.0 EXP. B			
VERT. ROOF PRESSURE			
Case A Cnw	Cnl	Case B Cnw	Cnl
1.20(20.31)	.30(5.08)	-1.10(-18.62)	-1.10(-1.69)
HORIZ. FASCIA PRESSURE EXP. B			
Case A & B	Cfx	Cfy	ASCE 7, FIG. 29.3-1
	1.92(35.42)	1.85(34.10)	
SEISMIC DATA			
2 SEC. SPECTRUM RESPONSE, Ss			0.1900 ASCE 7-16 FIG. 22.1
1 SEC. SPECTRUM RESPONSE, S1			0.0900 FIG. 22.2
LONG PERIOD TRANSITION PERIOD, Tl			12 FIG. 22-14
RISK CATEGORY			II TAB. 1.5-1
SEISMIC FACTOR, Ie			1.00 TAB. 1.5-2
SITE COEFFICIENT, Fa			1.60 TAB. 11.4-1
SITE COEFFICIENT, Fv			2.40 TAB. 11.4-2
SITE CLASSIFICATION			D TAB. 20.3-1
SITE ADJUSTMENT COEFFICIENT, Sms			0.3040 EQ. 11.4-1
SITE ADJUSTMENT COEFFICIENT, Sm1			0.2160 EQ. 11.4-2
DESIGN SPECTRAL RESPONSE, SDS			0.2028 EQ. 11.4-3
DESIGN SPECTRAL RESPONSE, SD1			0.1441 EQ. 11.4-4
W, Kips			12.99 12.8
SEISMIC RESPONSE COEFFICIENT, Cs			0.1622 0.1622 12.8.1.1
BASIC STRUCTURAL SYSTEM - SEISMIC RESISTING SYSTEM			
ORID. CANT. COL./ORID. CANT. COL.			
RESPONSE MODIFICATION FACTOR, R			1.25 1.25 TAB. 12.2-1
METHOD OF ANALYSIS - EQUIVALENT LATERAL FORCE			V=CqW 12.8
BASE SHEAR, Kips			2.63 2.63 EQ. 12.8-1

PREPARED BY: CARTER-MILLER-SANSING, LTD., P.O.B 4324, MERIDIAN, MS 39304, 601-483-0601

22'x108' CANOPY

SHADY VENT
315 2ND ST / CEDARTOWN, GA 30125
PHONE: 1-(770) 943-5977

NO.	DATE:	BY:
1		
2		
3		

CUSTOMER:	
SCALE: NOTED	DRAWN:
DATE:	APPROVED BY:
LOCATION:	REVISED:
DRAWING NO:	

CERTIFICATION